# AN IMPROVED THEORY OF THE VIBRATIONS OF A MULTILAYER ORTHOTROPIC PLATE $\dagger$ 

S. V. Kolesnikov<br>St Petersburg

(Received 17 November 1992)


#### Abstract

An improved theory of the vibrations of multilayer orthotropic plates of finite dimensions is proposed, based on the method of hypotheses $[1,2]$ and expansion of the normal displacements in powers of the plate thickness. The choice of the hypotheses is governed by the possibility both of using the results in practice and of solving new problems in the theory of the vibrations of an elastic rectangle.


Consider a cylindrical stiff plate consisting of an odd number $(2 m+1)$ of orthotropic layers arranged symmetrically about the middle layer, to which we assign the index zero ( $i=0$ ). Layers above the middle one are assigned positive indices from $i=1$ to $i=m$, and layers below it, negative indices from $i=-1$ to $i=m$. Layers symmetrical about the middle one have the same thicknesses and elastic parameters. The directions of the cylindrical system of coordinates $X, Y, Z$ are assumed to coincide with the principal anisotropic directions of the elastic material. The origin is situated at the half-thickness of the plate.
The main aim of this paper is to study the motion of the multilayer plate in the $X O Z$ plane, which cuts across the cross-section of the cylinder and is perpendicular to the $O Y$ axis. Our derivation of the equations of motion will be based on a two-dimensional formulation of the problem and the following hypotheses:

1. the differential equations of equilibrium and the components of the strain for a weakly bent plate will be based on the assumption that the stressed state of the curved plate is identical with the corresponding state for a flat plate, i.e. the curvature of the plate may be ignored in the equations of motion;
2. the shear stress $F_{x i}$ varies with the thickness of the multilayer plate in accordance with a given law [1, p. 46]

$$
\begin{aligned}
& F_{x i i}=\varphi(x, t) f(z), \quad f(z)=f(-z),\left.\quad f(z)\right|_{z= \pm h}=0 \\
& \varphi(x, t) \equiv \varphi, \quad F_{x z i} \equiv F_{x z}
\end{aligned}
$$

3. the normal displacement may be expanded in powers of the plate thickness

$$
\begin{aligned}
& U_{z i}=a x+\sum_{j=0}^{2 N+1}\left(\frac{z}{h}\right)^{j} W_{j}(x, t) \\
& W_{j}(x, t) \equiv W_{j}, \quad U_{z i} \equiv U_{z}
\end{aligned}
$$

where $x, z$ are curvilinear coordinates, $t$ is the time, $2 h$ is the thickness of the plate, $F_{x z i}$ and $U_{z i}$ are the shear stress and normal displacement in the $i$ th layer, $f(z)$ is a function characterizing
the variation of the shear stress with plate thickness, $\varphi$ is an unknown function, $a$ is the unknown angle through which the plate rotates as a whole, $W_{j}$ is the unknown component of the normal displacement in its expansion in powers of the plate thickness, and $2 N+1$ is the number of expansion terms.
The equilibrium conditions for the material of the $i$ th layer for harmonic variations in weakly curved cylindrical coordinates are given by the following differential equations [2, p. 18]

$$
\begin{align*}
& \partial \sigma_{x i} / \partial x+\partial F_{x i} / \partial z+\omega^{2} \rho_{i} U_{x i}=0 \\
& \partial \sigma_{z i} / \partial z+\partial F_{x i} / \partial x+\omega^{2} \rho_{i} U_{z i}=0 \tag{1}
\end{align*}
$$

where $i$ is the layer index ( $i$ varies from $-m$ to $m$ ), $\rho_{i}$ is the density of the layer material, $U_{x i}$, $U_{z i}$ and $\sigma_{x i}, \sigma_{z i}$ are the displacements and principal stresses in the layer in the direction indicated, $F_{x z i}$ is the shear stress, and $\omega$ is the angular frequency.

Substituting the given expressions for the shear and normal stresses (evaluated at the middle surface of the displacement) into the second equilibrium condition (1), integrating the result with respect to the normal coordinate $z$ from $z_{i}$ to $z$ (where $z$ lies within the $i$ th layer), and taking into account that the normal stresses at the layer faces must be equal, we find an expression for the normal component of the principal stress at any point of the multilayer plate

$$
\begin{align*}
& \sigma_{z i}=\sigma-J_{0}(z) \partial \varphi / \partial x- \\
& -\omega^{2}\left[a x\left(R Z_{i 0}+I_{i 0}(z)\right)+\sum_{j=1}^{2 N+1} W_{j}\left(I_{i j}(z)+R Z_{i j}\right)\right]  \tag{2}\\
& J_{i}(z)=\int_{z_{i}}^{2} f(z) d z, \quad I_{i j}(z)=\rho_{i} \int_{z_{i}}^{z}\left(\frac{z}{h}\right)^{j} d z \\
& R Z_{ \pm s, j}=R Z_{ \pm s \neq 1, j}+I_{ \pm s \mp 1, j}\left(z_{ \pm s}\right), \quad R Z_{0 j}=0, \quad Z_{0}=0 \\
& S=1,2, \ldots, m, \quad j=0,1, \ldots, 2 N+1
\end{align*}
$$

where $\sigma$ is an unknown constant of integration, $Z_{1}, Z_{2}, \ldots, Z_{m+1}$ are the coordinates of the upper faces of layers $0,1, \ldots,-m$, and $Z_{-1}, Z_{-2}, \ldots, Z_{-m-1}$ are those of the lower faces of layers $0,-1, \ldots,-m$.

Using (2), the fact that $f(z)$ is an even function and the symmetrical arrangement of the layers about the middle layer, we find an equation for the equilibrium of the plate and the constant of integration

$$
\begin{align*}
& \sigma_{2}-\sigma_{1}=Q_{11} \partial \varphi / \partial x-2 \omega^{2}\left[a x R Z_{m+1,0}+W_{0} R Z_{m+1,0}+W_{2} R Z_{m+1,2}+\ldots\right] \\
& \sigma_{2}+\sigma_{1}=2 \sigma-2 \omega^{2}\left[W_{1} R Z_{m+1,1}+W_{3} R Z_{m+1,3}+\ldots\right] \tag{3}
\end{align*}
$$

where $\sigma_{1}$ and $\sigma_{2}$ are the normal stresses at the lower and upper faces of the plate.
Differentiating the first equation of (3) with respect to $x$, we obtain the first equation of motion of the plate

$$
\begin{align*}
& \partial\left(\sigma_{2}-\sigma_{1}\right) / \partial x=Q_{11} \partial^{2} \varphi / \partial x^{2}+\omega^{2}\left(-a H_{1}+P_{12} \alpha_{0}+P_{13} \alpha_{2}+P_{14} \alpha_{4}+\ldots\right) \\
& \alpha_{j}=\partial W_{j} / \partial x, \quad W_{j}=C_{j}+\int \alpha_{j} d x, \quad P_{1 s}=-2 R Z_{m+1,2 s-4}  \tag{4}\\
& H_{1}=2 R Z_{m+1,0}, \quad S=2,3, \ldots, N+2, \quad j=0,1, \ldots, 2 N+1
\end{align*}
$$

where $\alpha_{j}$ is the unknown component of the angle of rotation of the plate in its expansion in powers of the thickness, and $C_{j}$ are unknown constants of integration.

The stressed state of the orthotropic material of layer $i$ is determined by the semi-inverse method of the theory of elasticity, using Hooke's law [1, pp. 21, 46]

$$
\begin{align*}
& \sigma_{x i}=B_{i} e_{x i}+A_{i} \sigma_{z i}, \quad B_{i}=\frac{E_{1}^{i}}{1-v_{12}^{i} v_{21}^{i}}  \tag{5}\\
& F_{x z i}=G_{i} e_{x z i}, \quad A_{i}=\frac{E_{1}^{i}}{E_{3}^{i}} \frac{v_{13}^{i}+v_{12}^{i} v_{23}^{i}}{1-v_{12}^{i} v_{21}^{i}}
\end{align*}
$$

where $e_{x i}, e_{x i j}$ are the volume and shear strains in the $i$ th layer, $v_{12}^{i}, v_{13}^{i}, v_{21}^{i}, v_{23}^{i}$ are Poisson's ratios, $E_{1}^{i}, E_{3}^{i}$ are Young's moduli and $G_{i}$ is the shear modulus.

Let us rewrite the last equation of (5) in terms of weakly curved cylindrical coordinates [2, p. 18], in the form

$$
\partial U_{x i} / \partial z+\partial U_{z i} / \partial x=G_{i}^{-1} f(z) \varphi(x, t)
$$

Integrating this equation with respect to $z$ from $z_{i}$ to $z$ (where $z$ is within the $i$ th layer) and taking into account that the displacements at the layer faces are equal, we obtain an expression for the tangential displacements at any point of the plate

$$
\begin{align*}
& U_{x i}=b+V-a z-\sum_{j=0}^{2 N+1} \frac{z}{(1+j)}\left(\frac{z}{h}\right)^{j} \alpha_{j}+\varphi\left(A I_{i}+J_{i}(z) / G_{i}\right)  \tag{6}\\
& V_{x 0} I_{z=0}=b+V, \quad V(x, t) \equiv V, \quad e_{x i}=\partial U_{x i} / \partial x \\
& A I_{ \pm s}=A I_{ \pm \leq \mp 1}+J_{ \pm s \mp 1}\left(z_{ \pm s}\right) / G_{ \pm s \mp 1}, \quad A I_{0}=0 \\
& S=1,2, \ldots, m+1
\end{align*}
$$

where $b$ is an unknown constant, equal to the tangential displacement of the plate as a whole, and $V$ is the tangential displacement of the middle surface due to the plate vibrations.

Substituting (6) into the first equation of (5), we obtain

$$
\begin{align*}
& \sigma_{x i}=B_{i} \frac{\partial V}{\partial x}-B_{i}\left(\frac{z^{2}}{2 h} \frac{\partial \alpha_{1}}{\partial x}+\frac{z^{4}}{4 h^{3}} \frac{\partial \alpha_{3}}{\partial x}+\ldots\right)+A_{i} \sigma- \\
& -\omega^{2} A_{i}\left[W_{1}\left(R Z_{i 1}+I_{i 1}(z)\right)+W_{3}\left(R Z_{i 3}+I_{i 3}(z)\right)+\ldots\right]- \\
& -B_{i}\left(z \frac{\partial \alpha_{0}}{\partial x}+\frac{z^{3}}{3 h^{2}} \frac{\partial \alpha_{2}}{\partial x}+\ldots\right)+\frac{\partial \varphi}{\partial x}\left[B_{i}\left(A I_{i}+\frac{J_{i}(z)}{G_{i}}\right)-A_{i} J_{0}(x)\right]-  \tag{7}\\
& -\omega^{2} A_{i}\left[a x\left(R Z_{i 0}+I_{i 0}(z)\right)+W_{0}\left(R Z_{i 0}+I_{i 0}(z)\right)+W_{2}\left(R Z_{i 2}+I_{i 2}(z)\right)+\ldots\right]
\end{align*}
$$

Using the first equation of equilibrium (5), Eqs (2)-(4), (7) and the fact that the moments acting over the cross-section of the plate are equivalent, we obtain the equations of the plate

$$
\begin{align*}
& \frac{\partial^{2}}{\partial x^{2}} D U+\omega^{2} R U=T \frac{\partial}{\partial x}\left(\sigma_{1}+\sigma_{2}\right)+\omega^{2} K b \\
& \frac{\partial^{2}}{\partial x^{2}} Q W+\omega^{2} P W=\left\|\begin{array}{c}
\partial\left(\sigma_{1}+\sigma_{2}\right) / \partial x \\
-4 \varphi / 3 \\
\vdots \\
-4 \varphi(2 N+3)^{-1}
\end{array}\right\|+\omega^{2} H a  \tag{8}\\
& D=\left(D_{j s}\right), \quad R=\left(R_{j s}\right), \quad T=\left(T_{j}\right), \quad K=\left(K_{j}\right)
\end{align*}
$$

$$
\begin{aligned}
& Q=\left(Q_{j s}\right), \quad P=\left(P_{j s}\right), \quad H=\left(H_{j}\right), \quad s, j=1,2, \ldots, N+2 \\
& \left|\begin{array} { c } 
{ M _ { 0 } } \\
{ \vdots } \\
{ M _ { 2 N } }
\end{array} \left\|\left.=\frac{\partial}{\partial x} D U\left|\begin{array}{c}
M_{1} \\
\vdots \\
M_{2 N+1}
\end{array} \|=\frac{\partial}{\partial x}\right| \begin{array}{ccc}
Q_{21} & \ldots & Q_{2, N+2} \\
\vdots & & \vdots \\
Q_{N+2,1} & \ldots & Q_{N+2, N+2}
\end{array} \right\rvert\, W\right.\right. \\
& U^{\prime}=\left(V, \alpha_{1}, \ldots, \alpha_{2 N+1}\right), \quad W^{\prime}=\left(\varphi, \alpha_{0}, \ldots, \alpha_{2 N}\right)
\end{aligned}
$$

where $U$ and $W$ are column-matrix functions, $U^{\prime}$ and $W^{\prime}$ are transposed matrix functions, and $M_{0}, \ldots, M_{2 N+1}$ are the moments of the zeroth, first, second, etc., orders. The explicit form of the matrix coefficients will be given at the end of the paper.

The motions of the plate are described by two systems of matrix wave equations-symmetric and antisymmetric vibrations. It is obvious from (8) that the number of equations in each linear system of differential equations and the number of boundary conditions are determined by the appropriate terms in the expansion of the normal stress in series in powers of the plate thickness. If only the zeroth term of this series is considered, one obtains the usual equation for the symmetric vibrations, while the system of equations for the antisymmetric vibrations differs from the usual equations only in its notation and in the unknown functions that have to be determined [1, p. 238]. The difference is that the equations of motion and approximate boundary conditions are written in terms of the unknown angles of rotation of the plate, not in terms of the unknown displacements. The new notation for the equations of motion and the boundary conditions is essential in studying vibrations of thick multilayer plates of finite dimensions, since in that case the operators describing the homogeneous matrix equations have adjoints [3, p. 413; 4, p. 20].

The general solution of problem (8) for plates of finite size may be sought as a series in column-matrix eigenfunctions [ $5, \mathrm{p} .78$ ], where the latter satisfy the homogeneous wave equations of the natural vibrations of the plate

$$
\begin{align*}
& \left(L_{s}+\omega_{n}^{1} R\right) U_{n}=0, \quad U_{n}^{\prime}=\left(V_{n}, \alpha_{1 n}, \ldots, \alpha_{2 N+1, n}\right) \\
& \left(L_{a}+\varepsilon_{l}^{2} P\right) W_{l}=0, \quad W_{l}^{\prime}=\left(\varphi_{l}, \alpha_{0 l}, \ldots, \alpha_{2 N, l}\right)  \tag{9}\\
& L_{s}=D \partial^{2} / \partial x^{2}, \quad L_{a}=Q \partial^{2} / \partial x^{2}, \quad O^{\prime}=(0, \ldots, 0) \\
& \left.\left\|\begin{array}{c}
M_{0 n} \\
\vdots \\
M_{2 N, n}
\end{array}\right\|=\frac{\partial}{\partial x} D U_{n}\left\|\begin{array}{c}
M_{1 l} \\
\vdots \\
M_{2 N+1, l}
\end{array}\right\|=\frac{\partial}{\partial x} \right\rvert\, \begin{array}{ccc}
Q_{21} & \ldots & Q_{2, N+2} \\
\vdots & \vdots \\
Q_{N+2,1} & \ldots & Q_{N+2, N+2}
\end{array} \| W_{l}
\end{align*}
$$

Here $O$ is a column of zeros, $n$ and $l$ the mode numbers of the natural vibrations of the plate, $\omega_{n}$ and $\varepsilon_{l}$ are the natural angular frequencies, $U_{n}$ and $W_{t}$ are the column-matrix eigenfunctions of symmetric and antisymmetric vibrations, $M_{0 n}, \ldots, M_{2 N, n}, M_{11}, \ldots, M_{2 N+1, t}$ are the moments of different orders due to the appropriate natural vibrations in the cross-section of the plate, and $L_{s}$ and $L_{a}$ are the matrix-valued differential operators of the symmetric and antisymmetric vibrations.

We will introduce an abbreviated notation for convolutions of matrix expressions

$$
\begin{equation*}
(y, z)=\int_{a}^{b} z^{\prime}(x) \bar{y}(x) d x=\int_{a}^{b} \bar{y}^{\prime}(x) z(x) d x \tag{10}
\end{equation*}
$$

where $y$ and $z$ are column matrices, $\bar{y}$ is a column matrix whose elements are the complex conjugates of those of $y$, and the prime denotes transposition.

Using (10) and applying Lagrange's formula [4, p. 20] to the matrix equations (9), we obtain

$$
\begin{aligned}
& \left(T, L_{s} U_{n}\right)=\left.\left(\bar{T}^{\prime} D \partial U_{n} / \partial x-U_{n}^{\prime} D^{\prime} \partial \bar{T} / \partial x\right)\right|_{a} ^{b}+\left(L_{s}^{*} T, U_{n}\right) \\
& \left(Y, L_{a} W_{l}\right)=\left.\left(\bar{Y}^{\prime} Q \partial W_{l} / \partial x-W_{l} Q^{\prime} \partial \bar{Y} / \partial x\right)\right|_{a} ^{b}+\left(L_{a}^{*} Y, W_{l}\right)
\end{aligned}
$$

It is obvious that, assuming homogeneous boundary conditions at the faces of the plate for the problem and its adjoint, the operators describing the plate vibrations have adjoints [3, 4], i.e.

$$
\left(T, L_{s} U_{n}\right)=\left(L_{s}^{*} T, U_{n}\right), \quad\left(Y, L_{a} W_{l}\right)=\left(L_{a}^{*} Y, W_{l}\right)
$$

( $L_{s}^{*}, L_{a}^{*}$ are the adjoint operators).

## APPENDIX

The matrix coefficients are

$$
\begin{aligned}
& D_{j 1}=\left.\Sigma \frac{2 B_{i}}{2 j-1}\left(\frac{z}{h}\right)^{2 j-1}\right|_{z_{i}} ^{z_{i+1}}, \quad R_{j 1}=\left.\Sigma \frac{2 \rho_{i}}{2 j-1}\left(\frac{z}{h}\right)^{2 j-1}\right|_{z_{i}} ^{z_{i+1}} \\
& D_{j s}=-\left.\Sigma \frac{B_{i}}{(s-1)(2 j+2 s-3)}\left(\frac{z}{h}\right)^{2 j+2 s-4}\right|_{z_{i}} ^{z_{i+1}}, \quad K_{j}=-R_{j 1} \\
& R_{j s}=\Sigma\left\{\frac{-\rho_{i} z}{(s-1)(2 j+2 s-3)}\left(\frac{z}{h}\right)^{2 j+2 s-4}-2 A_{i}\left(I_{i}\binom{2 j-2}{2 s-3}(z)+\right.\right. \\
& \left.\left.\left.+\frac{R z_{i, 2 s-3}}{2 j-1}\left(\frac{z}{h}\right)^{2 j-1}\right)\right)\right\}_{z_{i}}^{z_{i+1}}+\left.R Z_{m+1,2 s-3} \sum \frac{2 A_{i}}{2 j-1}\left(\frac{z}{h}\right)^{2 j-1}\right|_{z_{i}} ^{z_{i+1}} \\
& j=1,2, \ldots, N+2, \quad s=2,3, \ldots, N+2 \\
& Q_{11}=4 / 3 h^{3}, \quad P_{11}=0, \quad H_{1}=2 R Z_{m+1,0}, \quad P_{1 s}=-2 R Z_{m+1,2 s-4}, \quad Q_{1 s}=0 \\
& P_{j 1}=\left.\sum 2 \rho_{i}\left[\frac{J\binom{2 j-3}{i}(z)}{G_{i}}+\frac{A I_{i}}{(2 j-2) h}\left(\frac{z}{h}\right)^{2 j-2}\right]\right|_{z_{i}} ^{z_{i+1}} \\
& Q_{j 1}=\left.\sum 2\left\{B_{i}\left(\frac{J\binom{2 j-3}{i}(z)}{G_{i}}+\frac{A I_{i}}{2 j h}\left(\frac{z}{h}\right)^{2 j}\right)-A_{i} J\binom{2 j-3}{0}(z)\right\}\right|_{x_{i}} ^{z_{i+1}} \\
& Q_{j s}=-\left.\Sigma \frac{2 B_{i}}{(2 s-3)(2 j+2 s-5)}\left(\frac{z}{h}\right)^{2 j+2 s-5}\right|_{z_{i}} ^{z_{i+1}} \\
& P_{j ;}= \\
& =\left.\sum 2\left\{\frac{-\rho_{i}}{(2 s-3)(2 j+2 s-5)}\left(\frac{z}{h}\right)^{2 j+2 s-5}-A_{i}\left(f_{i}\binom{2 j-3}{2 s-4}(\mathrm{z})+\frac{R Z_{i, 2 s-4}}{(2 j-2) h}\left(\frac{z}{h}\right)^{2 j-2}\right)\right\}\right|_{z_{i}} ^{z_{i+1}}
\end{aligned}
$$

$$
\begin{aligned}
& H_{j}=\left.\sum 2 A_{i}\left(I_{i}\binom{2 j-3}{0}(z)+\frac{R Z_{i 0}}{(2 j-2) h}\left(\frac{z}{h}\right)^{2 j-2}\right)\right|_{z_{i}} ^{z_{i+1}} \\
& I_{i}\binom{2 s}{j}(z)=\int \frac{z^{2 s}}{h^{2 s+1}} I_{i j}(z) d z, \quad J\binom{2 s+1}{i}(z)=\int \frac{z^{2 s+1}}{h^{2 s+3}} J_{i}(z) d z \\
& I_{i}\binom{2 s+1}{j}(z)=\int \frac{z^{2 s+1}}{h^{2 s+3}} I_{i j}(z) d z, \quad s, j=2,3, \ldots, N+2
\end{aligned}
$$

Summation is from $i=0$ to $i=m$.

## REFERENCES

1. AMBARTSUMYAN S. A., Theory of Anisotropic Plates. Nauka, Moscow, 1987.
2. AMBARTSUMYAN S. A., General Theory of Anisotropic Shells. Nauka, Moscow, 1974.
3. KORN G. A. and KORN T. M., Handbook of Mathematics for Scientists and Engineers. McGraw-Hill, New York, 1968.
4. NAIMARK M. A., Linear Differential Operators. Nauka, Moscow, 1969.
5. BOLOTIN V. V. (Ed.), Vibrations in Engineering. A Handbook in 6 Volumes. Vibrations of Linear Systems, Vol. 1. Mashinostroyeniye, Moscow, 1978.
